

SLURRY SEAL DESIGN FOR:
MARYLAND DEPARTMENT OF TRANSPORTATION
STATE HIGHWAY ADMINISTRATION
CONTRACT NO. D-542-501-177
"RESURFACE ROADWAY & SHOULDERS USING SLURRY SEAL ON MD SR16"

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DATE: MAY 11, 1990

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MATERIALS

AGGREGATE

Received from client April 25, 1990 four, 5 gallon pails of a Type 3 aggregate identified as Arundel Havre de Grace Diabase/Granite TRAP ROCK. Our Lab No. 266.

GRADATION

PERCENT PASSING

SIEVE#	SURFACE AREA FACTOR	0/#4 USED (DRY)	0-3/8" FOUND (DRY)	WASHED	SA	SPECS Md. "C"
3/8	.02	100	100	100	2.00	100
1/4	-	-	95.4	94.9	-	-
#4	.02	100	85.7	85.5	1.71	70-90
8	.04	73.4	62.9	60.7	2.43	45-70
16	.08	53.7	46.0	43.8	3.50	32-54
30	.14	38.6	33.1	32.2	4.51	23-38
50	.30	25.7	22.0	23.1	6.93	16-29
100	.60	15.8	13.5	16.0	9.60	9-20
200	1.60	9.1	7.8	11.0	17.60	5-15
325	-	2.7	2.30	-	-	-

Simple Surface Area
Corrected SA (2.65/2.75)

48.28
46.52ft²/lb.

Sand Equivalent
Methylene Blue Absorbtion mg/g
0/#325
Blue Factor
pH 10:1
Specific Gravity Dry
Specific Gravity SSD
Absorbtion %
Unit Weight, Loose 1.577 (98.42)
Unit Weight, Compacted 1.999 (124.09)
Compacted Voids % 27.7
Loose Voids % 42.7
Total Liquids Capacity
Loose, % added 27.0
Reaction w/HCl

73.0
4.0 mg
10.4
8.96-8.22
2.74-2.75*
2.71 (2.40)
2.90
1.652 (103.09)
2.012 (125.55)
26.83
39.92
24.15
Very Slightly Soluble

*Quarry Reports 2.80. We note that larger particles seemed light.

MOISTURE CONTENT BULKING EFFECT, DRY BASIS:

<u>% Moisture</u>	<u>Wt. Compacted</u>	<u>Wt. Loose</u>	
0	125.6	103.1	(lbs./CF)
.8	119.0	97.3	
1.6	115.4	86.3	
4.3	111.8	74.6	
6.2	115.0	77.5	

ASPHALT EMULSION

Received from client April 25, 1990 two, 1-gallon bottles identified as Slurry Pavers CSS1-h 4-23-90 T-2. Our Lab No. 00425-1.

<u>TEST</u>	<u>FOUND</u>	<u>SPECS</u>
Sieve, %	Trace	0.10 MAX
Residue, %	65.36	57.0 MIN
pH	3.36	-
R&B Softening Pt.	-	-

CHEMICAL FILLERS

Received from client August 29, 1989 identified as "LIME" (Hydrated) APG Lime, Inc. Ripplemead, VA.

Hydrated Lime, National Limestone Co. Findlay, OH
Identified as "DOLOMITIC HYDRATED LIME" (30%MgO) Current production 4/28/90

Type I Portland Cement from Southwestern Portland Cement Co., Fairborn, OH Current production.

K-3 Proprietary Additive, 1% solution

WATER

Softened well water, 556 N. Valley Rd. Xenia, OH.

**TRIAL MIXES FOR MIXING & SETTING CHARACTERISTICS,
WET COHESION, CURED COHESION & COMPATABILITY**
ISSA Technical Bulletins 102, 113, 114, 115, 139

Wet Cohesion - 0/#4 #266 Aggregate vs. 00425-1 AE

Formula #	Formula, %			AE	MIX" SET'	COHESION - kg-cm			APPEAR	ADHESION		
	FLR	H2O				30'	60'	120'		(1)	(2)	(3)
82	0	9		12	10 BUST	-	-	-		-	-	-
83	0	12		12	25 BUST	10.2	10.9	-		-	-	-
84	.25pc	18		12	180+ 10	10.0	10.9	-	B1 Tf	E	G	98
85	.5pc	16		12	sl wet 180+ 5	9.9	11.0	-	B1 Tf	E	G	-
86	.75	12		12	180+ 5	8.0	11.0	-	B1 Tf	E	G	-
87	1.0	9		12	160 3	9.0	11.0	-	B1 Tf	G	P	98
88	1.5	9		12	150 5	9.0	12.0	-	B1 Tf	G	P	-
89	2.0	10		12	160 4	9.5	9.2	-	D1 Tf	G	F	-
90	3.0	11		12	160 5	9.0	11.5	-	D1 Sl cr	G	P	98
91	.25hl	10		12	100 -	13.6	13.2	-	D1 Tf	G	G	-
92	.5	12		12	100 -	15.3	14.7	-	D1 Br Tf	G	G	-
93	.75	15		12	foamy 180+ -	15.0	14.5	-	D1 Tf	G	F	-
94	1.0	16		12	180+ -	13.6	14.2	-	D1 Tf	G	F	-
95	.25hl	10		15	180+ -	17.9	17.2 s	-	B1 Tf	G	G	-
96	.5hl	12		15	foamy 180+ -	15.9	16.7	-	Rch Tf	E	E	98+
97	.75hl	12		15	180+ -	13.9	18.0	-	Rch Tf	G	G	99+
104	0	12/1k3		12	20 BUST	-	-	-	-	-	-	-
105	0	14/4k3		12	60 -	20.2	21.0 s	20.0s	Rch Tf	E	E	-
106	.25hl	9/1k3		12	100 -	18.1	16.2	20.0	B1 Tf	G	G	-
107	.25	7/4k3		12	100 -	16.6	17.2	19.5	B1 Tf	G	G	-
108	.5	12/1k3		12	100 -	17.6	18.0	18.0	B1 Tf	G	G	-
109	.5	10/4k3		12	100 -	15.0	17.0	18.3	Sl Sz Tf	F	G	-
110	.25	10		12	100 -	15.5	17.0	15.0	B1 Tf	F	G	-
111	.5	12		12	100 -	15.0	16.9	18.9	B1 Tf	F	G	-
112	.15	10		12	100 -	-	18.0ns	18.2	B1 Tf	G	G	-
113	.15	9/1k3		12	110 -	-	17.0	21.3ns	B1 Tf	G	E	99+
114	.15	10		15	180+ -	-	20.0ns	24.2s	Rch Tf	E	E	98+
115	.15	9/1k3		15	180 -	-	21.8s	22.0nss	B1 Tf	Vg	Vg	-

12 kg-cm = "Set"
20 kg-cm = "Early Rolling Traffic"

ns = Near Spin
s = Spin
nss = Near Solid Spin
ss = Solid Spin

(1) = Wet Surface Adhesion
(2) = Substrate Adhesion
(3) = Boiling Water Adhesion - ISSA TB 114

60C CURED COHESION ISSA TB No. 139

				<u>1</u>	<u>2</u>	<u>3</u>				
98	.25 hl	10	12	15.0	17.5	17.1	16.5	Rch	Tf	99+
99	.5	12	12	13.9	17.0	18.0	16.3	Rch	Tf	99+
100	.75	14	12	18.2	14.9	17.0	16.7	Rch	Tf	99+
101	.25	10	15	18.7	17.2	16.9	17.6	Rch	Tf	
102	.5	11	15	17.5	18.4	16.9	17.6	Rch	Tf	
103	.75	12	15	16.0	18.5	18.8	17.8	Rch	Tf	
141	.15hl	10	12	16.2	14.0	13.8	14.7	Rch	Tf	
142	.15	9/1K3	12	21.2	16.2	18.8	18.7	Rch	Tf	
143	.15	7/4K3	12	18.2	21.0	21.3	20.2	Rch	Tf	
144	.15	9	15	19.2	20.9	21.3	20.5	Rch	Tf	
145	.15	8/1K3	15	21.2	19.2	21.8	20.7	Rch	Tf	
146	.15	6/4K3	15	20.3	22.5	24.0	22.3	Rch	Tf	

SCHULZE-BREUER-RUCK COMPATIBILITY CLASSIFICATION, ISSA TB No. 144

--0/#10 AGGREGATE GRADATION - AVERAGE OF 4 SAMPLES EACH:

		<u>Density</u>	<u>Absorbtion</u>	<u>Loss</u>	<u>Adhesion</u>	<u>Integrity</u>	<u>Grade</u>
			g.	g.	%	%	
A(4)	0-16-12.5	2.15	.69	.14	99+	98.2	AAA=12
B(4)	.15 hl 10-12.5	2.16	.66	.34	99+	98.5	AAA=12
C(4)	.15hl 10/1K3-12.5	2.16	.81	.34	99+	98.3	AAA=12

CONSISTENCY ISSA Tech. Bull. 106

Consisting tests to determine total liquids - mix viscosity characteristics were not performed; the relatively short mix times and quick setting characteristics at the optimum set and traffic times precluded accurate results from the consistency test. However, the total liquids capacity of both the 0-3/8" type 3 and 0/#4 type 2 gradations were determined at 24.15% and 27.04% respectively. Mixes at estimated consistencies of 3.0 to 3.5 cm. were easily achieved at optimum filler contents at less than the total liquids capacity of the loose aggregate.

FIELD SIMULATION TESTS

WET TRACK ABRASION TEST: ONE HOUR SOAK

	FLR	H2O	AE	Before g	After g	Loss g	Loss, g/ft ² (f=3.06)
1A	.15h1	11/1K3	9	767.9	751.5	16.4	
1B	.15	11/1K3	9	760.0	748.1	<u>11.9</u>	
				AVERAGE:		14.5	43.30
2A	.15h1	10/1K3	12	837.3	825.9	11.4	
3A	.15h1	10/1K3	12	829.4	819.4	<u>10.0</u>	
				AVERAGE:		10.7	32.74
3A	.15h1	8.5/1K3	15	854.5	846.8	7.7	
3B	.15h1	8.5/1K3	15	799.1	792.8	<u>6.3</u>	
				AVERAGE:		7.0	21.42

WET TRACK ABRASION TEST: SIX DAY SOAK

1C	.15h1	11/1K3	9	792.7	779.7	13.0	39.8
2C	.15h1	10/1K3	12	808.0	797.8	10.2	31.2
3C	.15h1	8.5/1K3	15	813.4	808.7	4.7	14.4

MONOLAYER LOADED WHEEL TEST, C109 FINE SAND ADHESION - 0/#4
ISSA TECHNICAL BULLETIN 109

COMPACTED 1000, 125 pound LWT cycles:

			<u>Before</u>	<u>After</u>	<u>Sand g</u>	<u>Sand.g/ft²(f=6.6)</u>
1b	.15h1	11/1K3	9 320.46	325.81	5.35	35.3
2b	.15h1	10/1K3	12 314.67	320.97	6.30	41.6
3b	.15h1	8.5/1K3	15 322.62	329.50	6.88	45.4

UNCOMPACTED

1a	.15h1	11/1K3	9 306.24	314.12	7.88	52.0
2a	.15h1	10/1K3	12 322.90	331.38	8.48	56.0
3a	.15h1	8.5/1K3	15 343.22	352.50	9.28	61.2

MULTILAYER LOADWHEEL SAND ADHESION - 0-3/8" AGG.
 (COMPACTED SAMPLE FROM MULTILAYER DISPLACEMENT TEST)

4	.15h1	11/1K3	9 559.37	566.00	6.63	43.8
5	.15h1	10/1K3	12 574.67	582.02	7.35	48.5
6	.15h1	8.5/1K3	15 584.04	592.32	8.28	54.7

MULTILAYER UNCONFINED LOADED WHEEL DISPLACEMENT TEST
 ISSA TECHNICAL BULLETIN 147A

1/2" Specimen Thickness - 0/#4 agg:

			% Displacement		<u>Compacted Density</u>	Remarks
			<u>Vertical</u>	<u>Lateral</u>		
1	.15h1	11/1K3	9 24.9	9.4	2.29	Ends & Edge Split
2	.15h1	10/1K3	12 22.2	9.2	2.26	Sl Ends Split, Sl Edge Split
3	.15h1	8.5/1K3	15 29.5	11.7	2.42	OK - Sl Rich

1/2" Specimen Thickness - 0-3/8" Agg:

4	.15h1	11/1K3	9 21.8	9.0	2.25	End Split, Sl Edge Split
5	.15h1	10/1K3	12 20.3	6.7	2.30	One End Split, Sl Rich
6	.15h1	8.5/1K3	15 20.2	9.8	2.31	OK - Rich

SURFACE AREA CALCULATION ISSA TECHNICAL BULLETIN #118
 (US ARMY, WES "INSTRUCTION REPORT S-75-1")

Corrected surface area (washed) is 46.52ft²/lb.
 Assume bitumen specific gravity 1.03
 Assume CKE(Centrifuge Kerosene Equivalent)(2.90 Water Absorbtion)
 (e2.40 CKE)

FORMULA: BR = SAB + KA
 SAB= CSA x t x 0.02047 x SGB
 WHERE: BR = Bitumen Requirement
 SAB= Surface area bitumen, % agg. dry weight
 CSA= Corrected surface area (2.65/2.75)
 t = Bitumen film thickness, microns
 SGB= Specific gravity of bitumen
 KA = Centrifuge kerosene equivalent (absorbtion)
 THEN: BR = (CSA x t x 0.02047 x SGB) + KA
 BR = (46.52 x 6.5 x 0.02047 x 1.03) + 2.90 [2.40]
 = 6.375 + 2.90 [2.40]
 = 9.275 [8.775] Bitumen added to aggregate
 = 8.488 [8.07]% Bitumen in total mix
 = 12.97% [12.34]% emulsion (@65.4% residue)
 included
 = 14.27 [13.42]% emulsion (@65.4% residue) added

VOIDS CALCULATION

Compacted #266 0-3/8" aggregate voids are 26.8% at a compacted unit weight of 125.55 lbs/ft³ (2.012) and dry specific gravity of 2.75. Bitumen requirement of 8.775% (from surface area calculation) which, at 1.03 sp.gr., is 8.52% water equivalent or 8.52/62.4 = 13.65 volume%

Unit Volume	100.0%
less Agg. Volume	<u>73.2%</u>
=Voids Volume	26.8%
less Bitumen Volume	<u>13.7</u>
=Voids in Total Mix	13.1
% Voids Filled	48.9
Agg Weight	125.6
Bit Weight	<u>11.0</u>
	136.6 lbs/ft ³

DISCUSSION

Materials submitted for this design met the requirements for the Maryland DOT specification in so far as tested.

Trial mixes for estimating total liquids contents, mixing characteristics, set and traffic time wet cohesion tests were performed using a series of chemical filler and additive contents. Type I Portland cement retarded the set to greater than one hour and would not meet the 2-hour minimum traffic time requirement. Use of Portland cement is rejected.

Mixes could not be made without either cement, hydrated lime or retarder. Small amounts of lime with low levels of retarder produced satisfactory mixes. .15% hydrated lime with 1% K3 retarder produced the required 2 minute mix times and 2-hour traffic times at 12% emulsion contents and total liquid contents of less than loose aggregate voids or liquid capacity.

60°C cured cohesion tests are improved with the addition of retarder. Boiling water adhesion is quite good in all cases. Hydrated lime above .5% is not recommended because of slight tackiness. The system selected for testing is compatible.

Schulze-Breuer-Ruck Compatibility tests were all excellent. The results found are a rare occurrence in our experience for unmodified systems. Each of the three formulations tested were rated AAA or 12 grade points; the best possible. These results confirm our trial mix compatibilities.

For design purposes, we selected .15% hydrated lime with 1% of K-3 additive.

Both the one hour and six-day soak Wet Track Abrasion Tests were excellent. Both tests yielded nearly identical results. The six-day soak test was slightly better than the one-hour soak indicating excellent wet weather adhesion. A reasonable minimum emulsion content cannot be established here with these WTAT test results.

The Monolayer Loaded Wheel fine sand adhesion test applied to the 0/#4 aggregate fraction indicates maximum emulsion content for this fraction of 15.9% AE. Corrected for 0-3/8" (85% 0/#4) the indicated maximum emulsion content is 13.5%.

The Multilayer Loaded Wheel Displacement tests for the 0/#4 fraction and 0-3/8" aggregate are greater than our recommended multilayer limits of 10-12% vertical and 5% lateral displacements. Projected compacted densities also substantially exceeded our limits for multilayered applications of 2.10-2.15.

However, maximum stabilities do occur at about 12% AE. This system while quite suitable for monolayer application is not recommended for multilayer applications such as rut filling.

Surface area designs as practiced are frequently unreliable, but we performed a surface area design for 6.5 micron coatings for confirmation. Our results based on washed gradation indicated an optimum emulsion content of 13.42% AE, which conforms to our found 13.5%AE from the corrected LWT 0/#4 sand adhesion test (above).

Voids calculation at 13.42% AE added yields 13.1% voids in the total compacted mix and 48.9% voids filled for a compacted unit weight of 136.6 lbs/ft³. These values are in line with general field experience.

Finally, we subjected the Compacted Multilayer Loaded Wheel 0-3/8" specimens to the fine sand adhesion test. Based on these test results, the maximum emulsion content for heavy traffic is 13.0% AE added to the aggregate.

We recommend a **MAXIMUM** emulsion content of 13.0% with a field tolerance of $\pm 1.1\%$; or an **OPTIMUM** of $11.9\% \pm 1.1\%$ AE (.75%AC).

JOB MIX FORMULA RECOMMENDATION

Aggregate	100%
Hydrated Lime	.15% (0-.5%)
K-3 Additive (1% Solution)	1% (.5-4%)
CQS1h Emulsifion (65.4%AC res)	11.9% \pm 1.1%
AC Extracted from Total Dry Mix	7.8% \pm .75%
Water	not to exceed 24% total liquids

SPREAD RATE

The spread rate for normal surfaces is estimated by ISSA Technical Bulletin No.112 at 18.9 ± 3 lbs/SY.

Please call if there are questions.

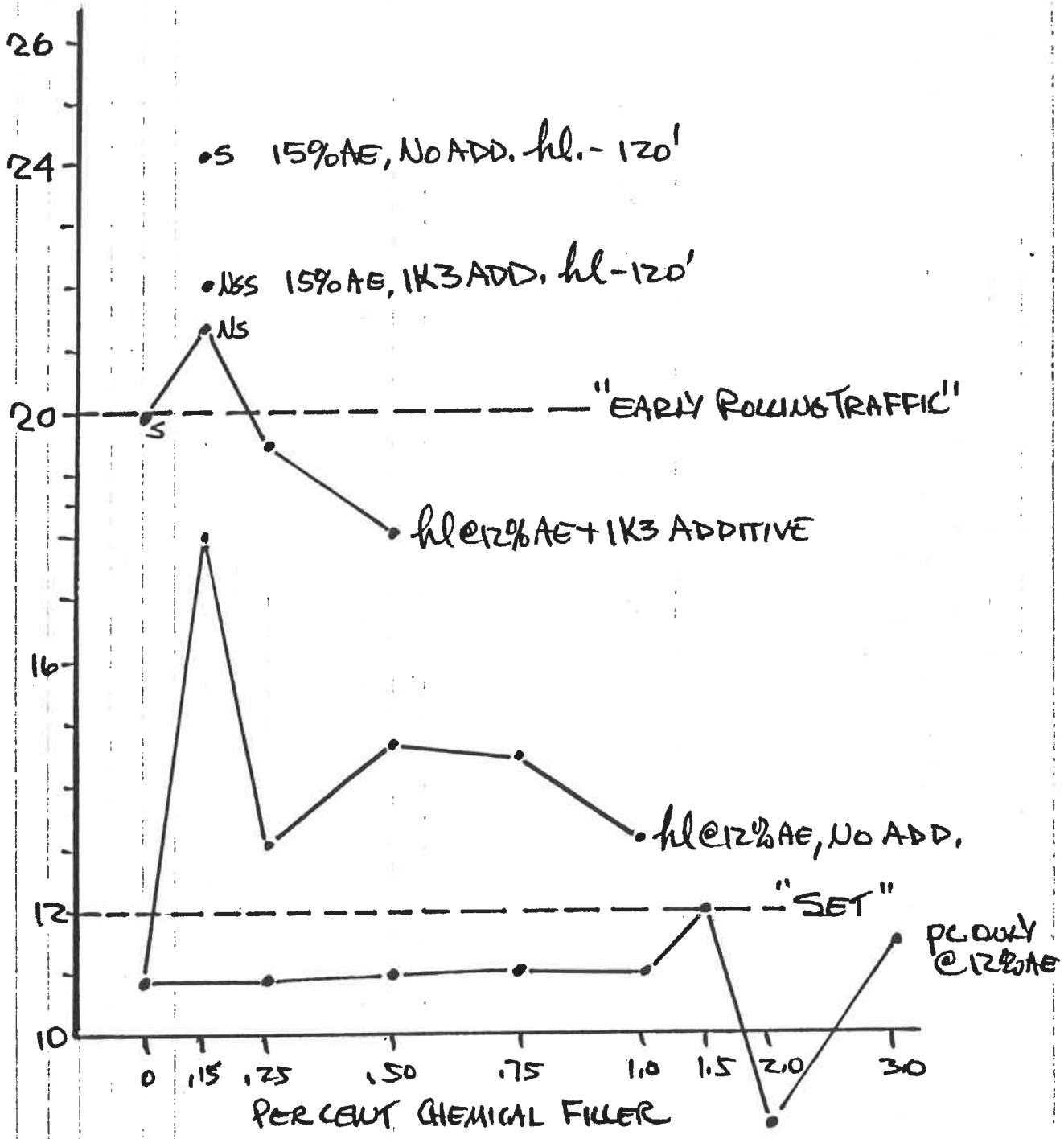
Respectfully submitted,

C. Robert Benedict, Consultant

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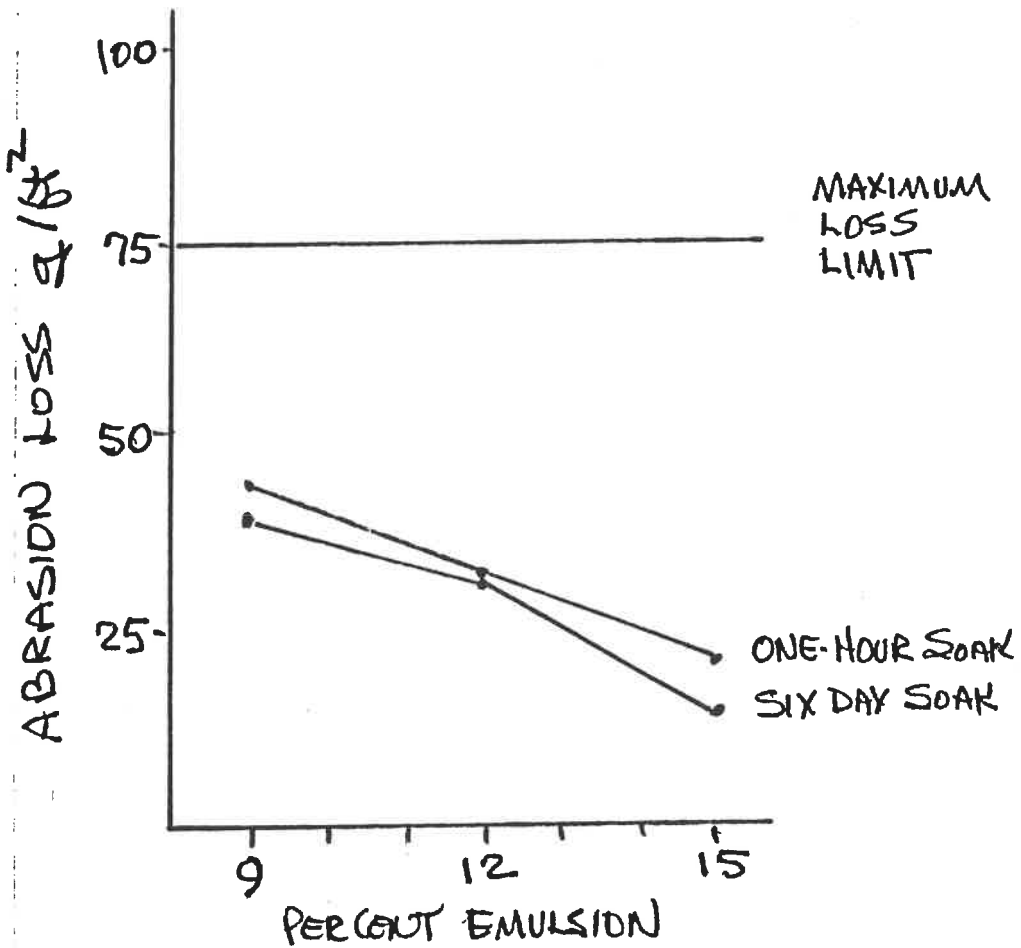
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60 MINUTE WET COHESION FOR
 FILLER-ADDITIVE & TRAFFIC TIME
 OPTIMIZATION

AE# 00425-1 NS. A66. # 266

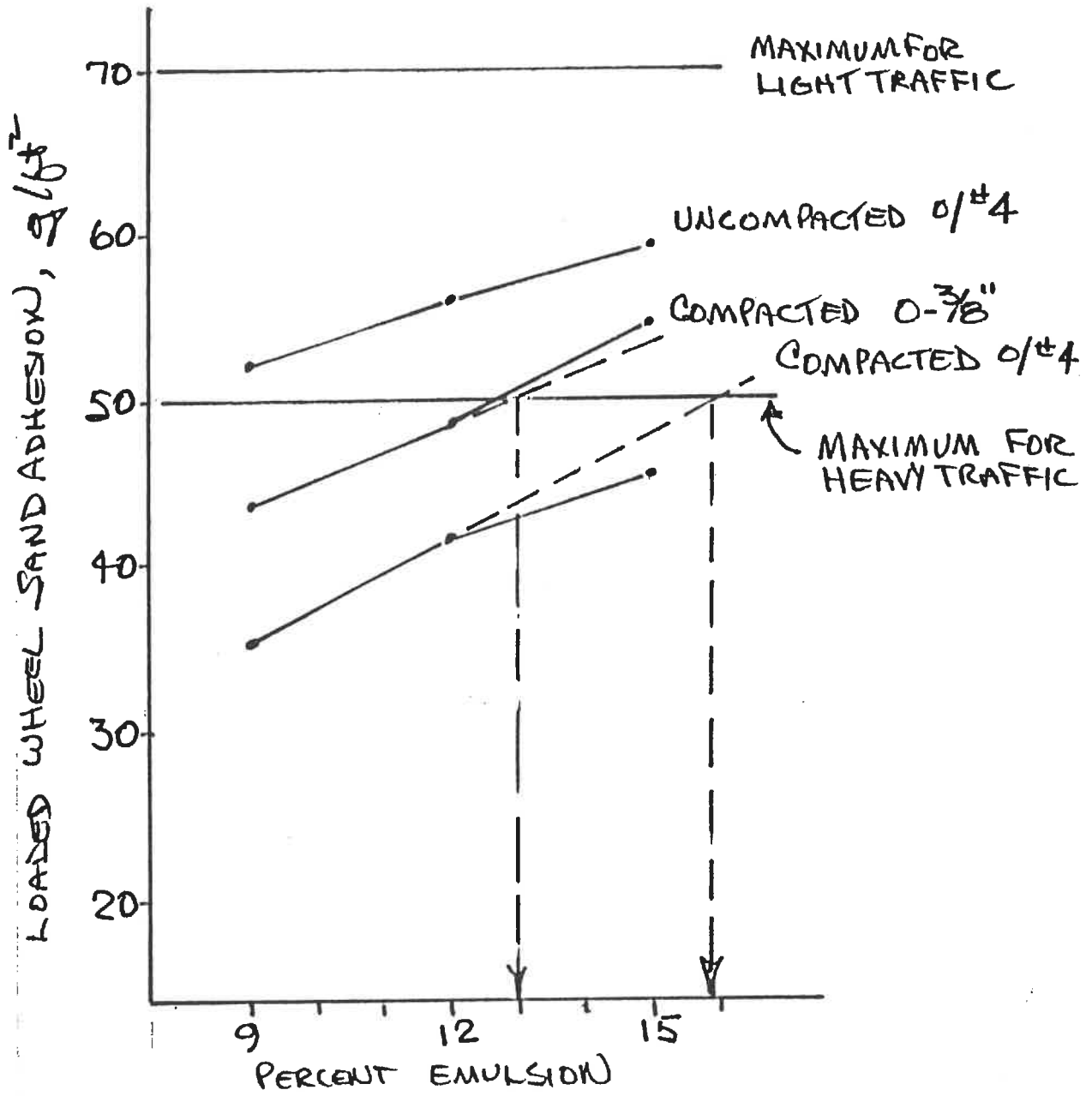
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WET TRACK ABRASION TEST

AE#00425-1 CASIK vs AGG#266

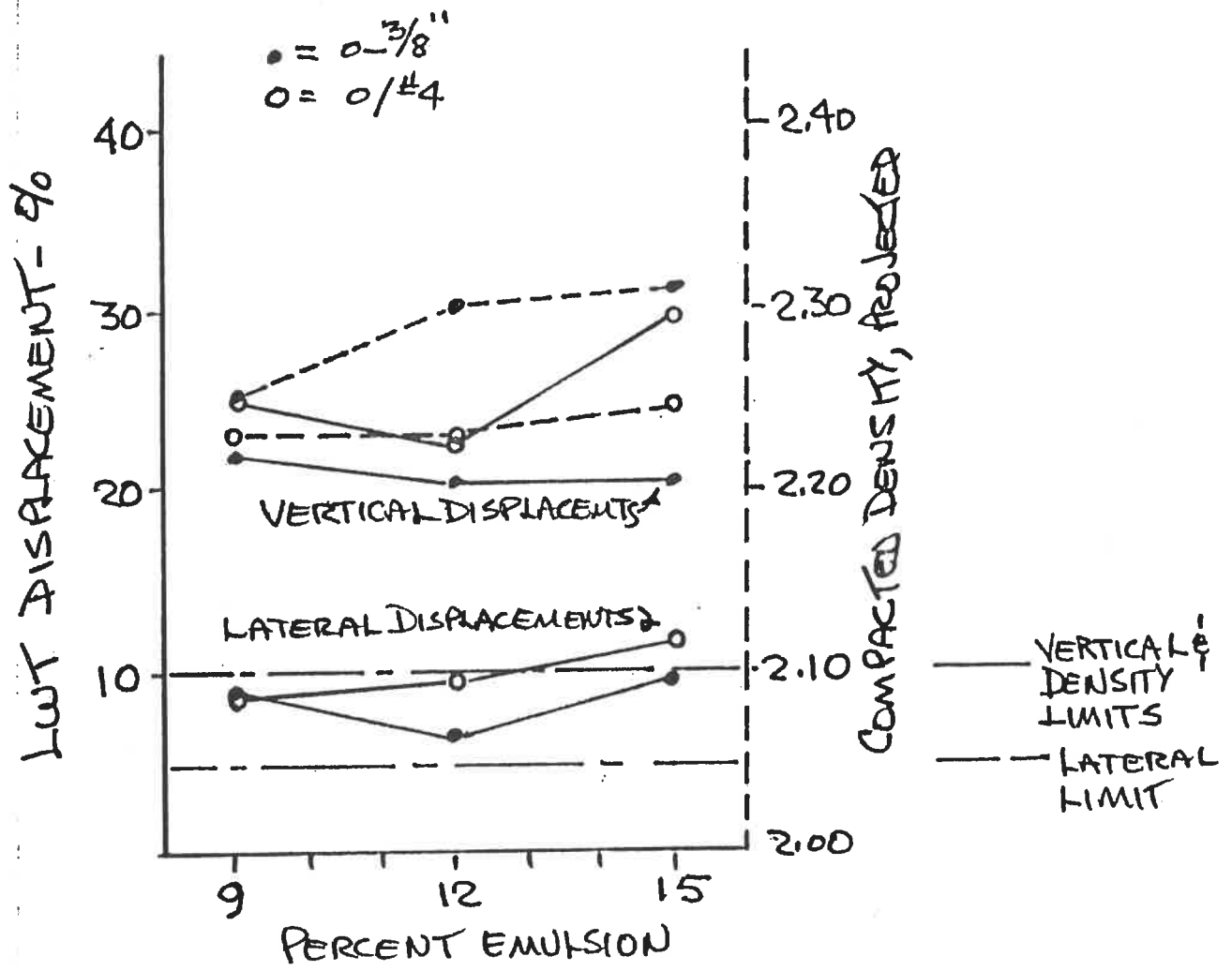
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LOADED WHEEL FINE SAND ADHESION TEST

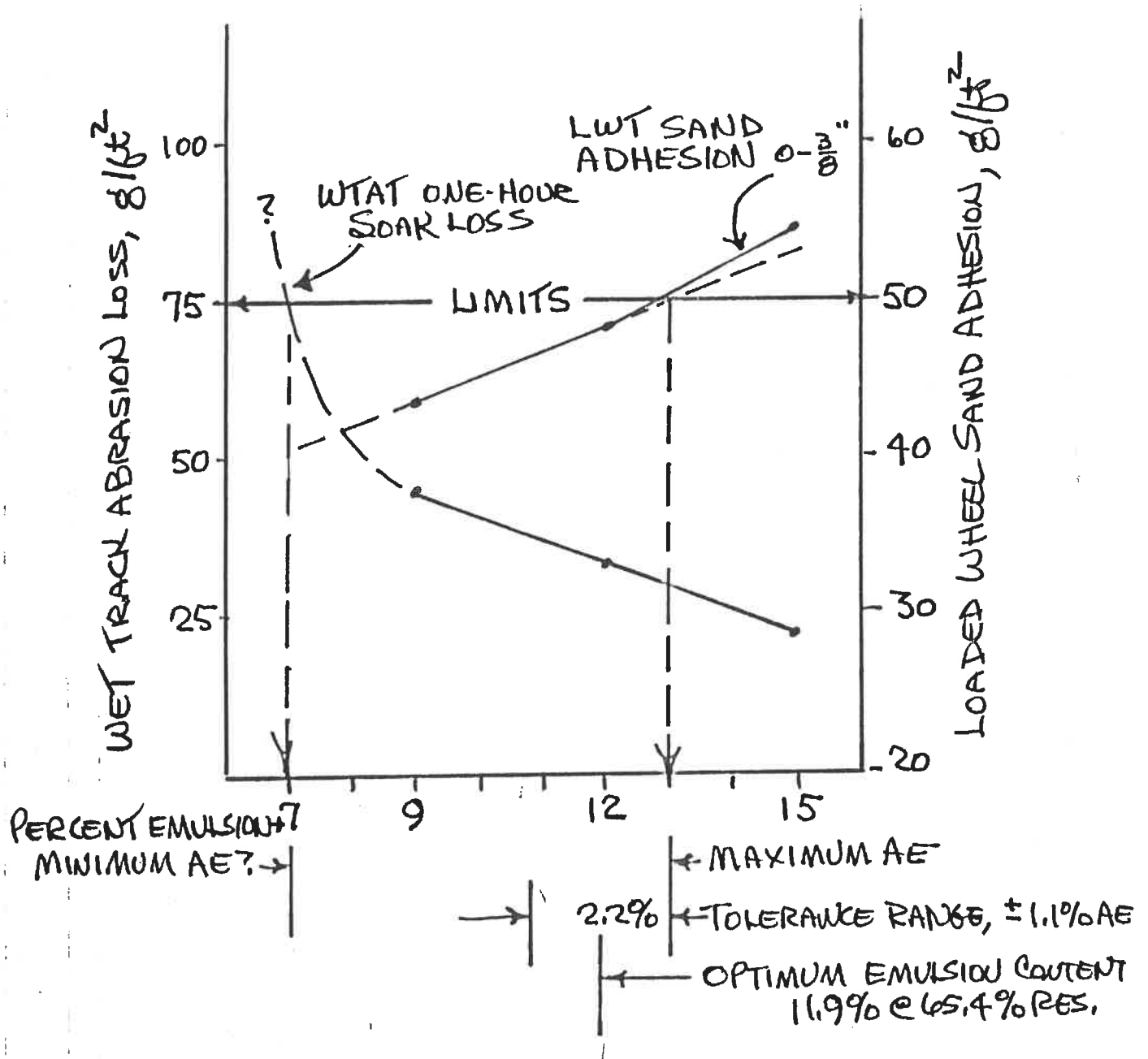
AE# 00425-1 CQSik M, #266 A66,

5/11/90-CRB



MULTILAYER LOADED WHEEL
 DISPLACEMENT TEST
 AE # 00425-1 CRSI # 266 AGG,

5/11/90-CRB



GRAPHICAL DETERMINATION OF OPTIMUM EMULSION CONTENT

AE#00425-1 CQS11ws. ABB.#266, 0-3/8"

5/11/90 - CPB

Assuming a moderate squeegee contact pressure, a slurry consistency of 2.5 to 3 cm. and a slurry depth of 5 to 6 inches, basic spread rates applied to smooth surfaces may be selected from the 1st table. The quantity of slurry required to fill surface texture may be added to the basic rate along with an estimate of requirements due to cross sectional irregularity and for joint cracks and laps. These increments may be added to give an estimate of the spread rate.

APPROXIMATE SPREAD RATE CALCULATION*
(Under study In 1977...Subject to revision)

120
130
56.2

61 610
-50-500
11 4.9
4.8 = 4 316
11
15.5
-14.0
1.5 4936
16.5
15.5
-1.0
14.85

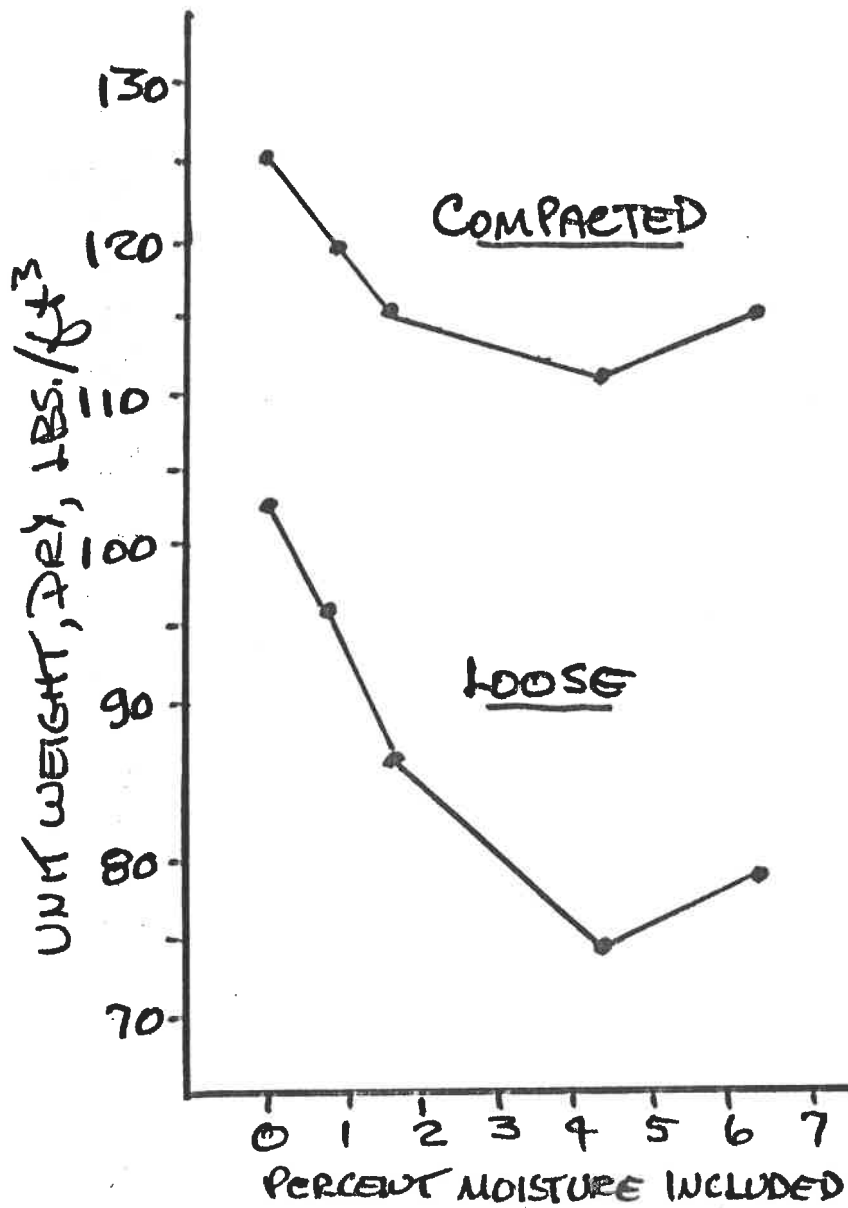
GRADATION	TYPE I		TYPE II		TYPE III	
	%+16	lb/SY	%+16	lb/SY	%+16	lb/SY
FINE	10	5	30	9	50	14
MEDIAN	22.5	6	42.5	10.5	61	15.5
COARSE	35	7	55	12	72	17

FACTORS:	McLeod Rating	Sand Box Texture	Add lb/SY	TOTAL
BASIC RATE	S	16-18'		14.9
ADD FOR SURFACE TEXTURE	H-1	10-12'	1	2
	H-2	8-10'	2	
	H-3	5-7'	3	
		2-4'	4	
ADD FOR CROSS SECTIONAL IRREGULARITY	Nominal - 3/8"		1	1
	Moderate - 1/2-3/4"		2	
	Severe - 1-1-1/2"		3	
ADD FOR JOINT CRACKS & LAPS (Calculate)				1
APPROXIMATE SPREAD RATE - TOTAL				18.9

+ 3# / SY

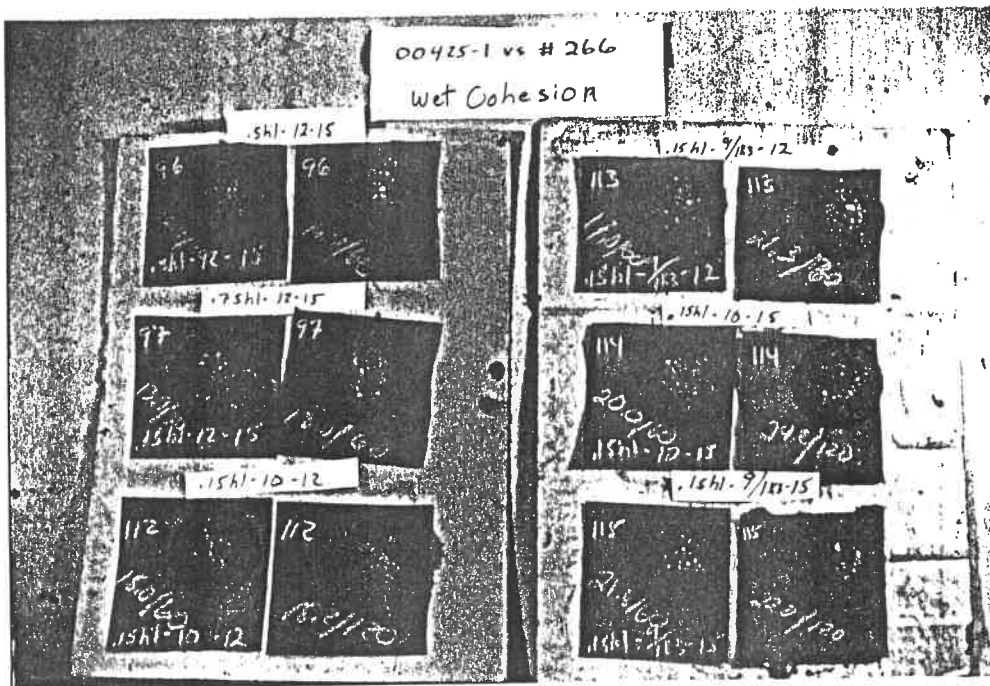
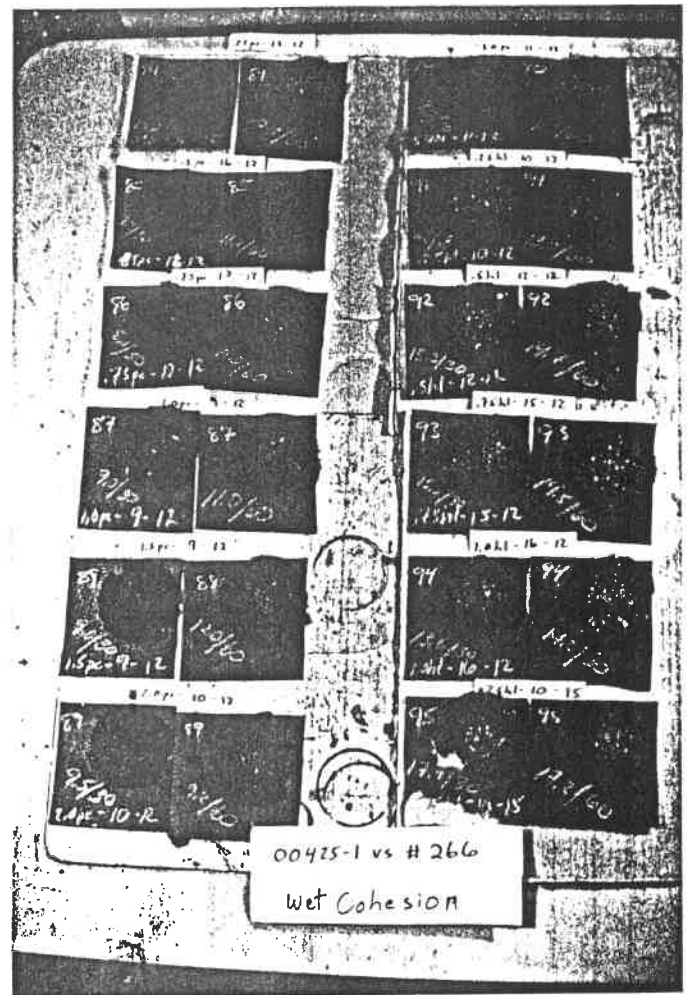
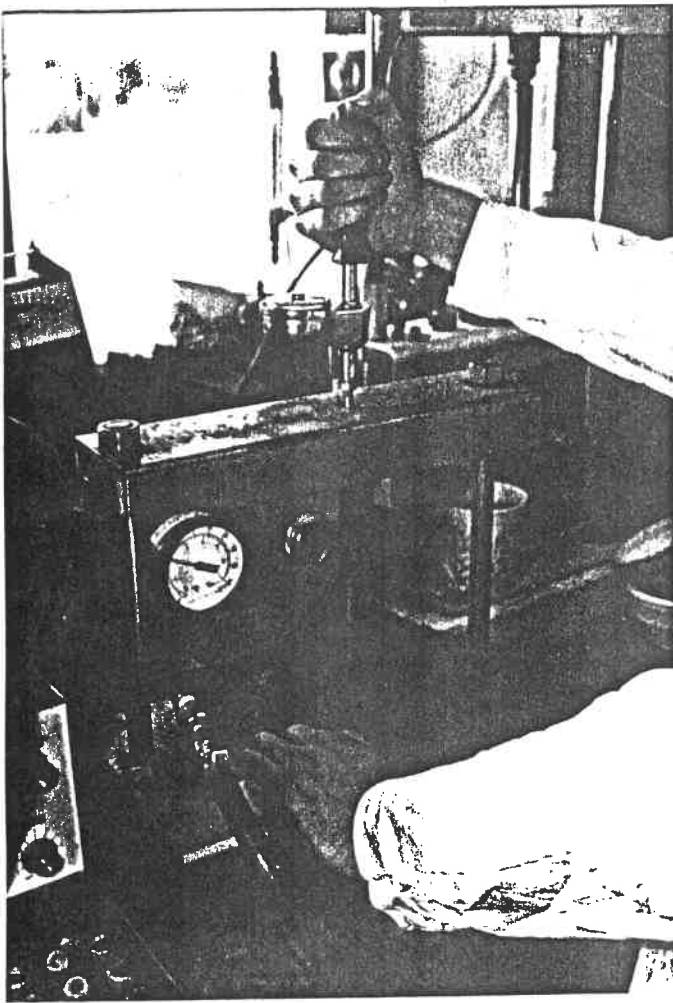
* Variables of Particle Shape, Dimensions, Matrix Volumes, Void Content, Screen Ratios, All Affect the Spread Rate. Use these tables as a GUIDE only.

16-

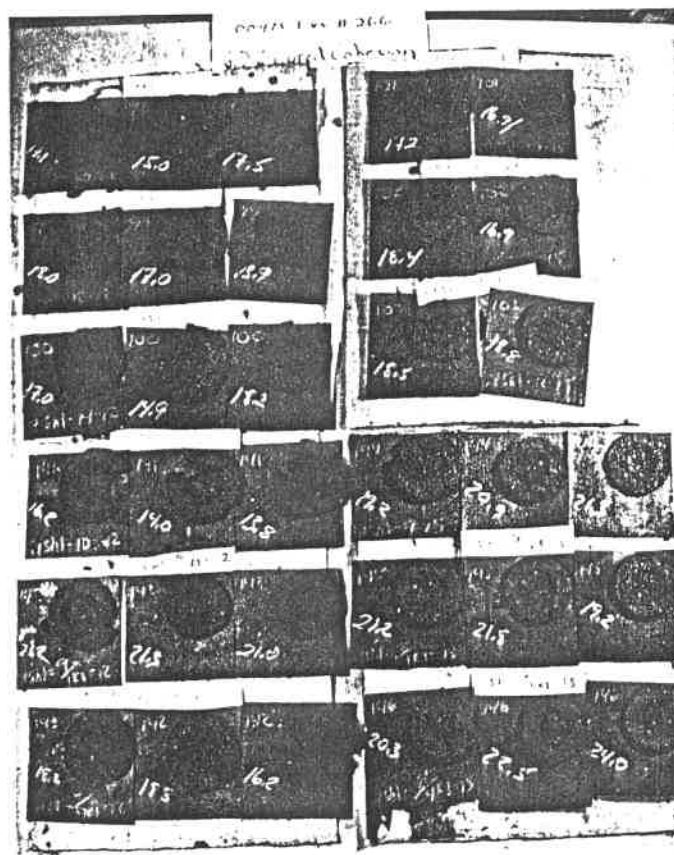
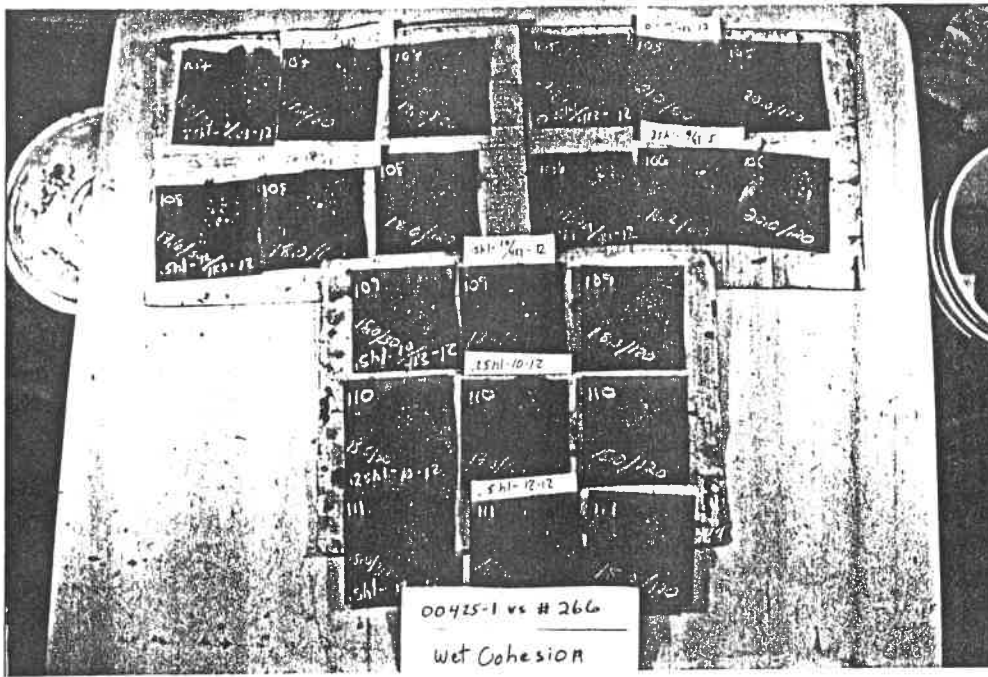


BULKING-EFFECT OF MOISTURE CONTENT
 #266, 0- $\frac{3}{8}$ " AGGREGATE

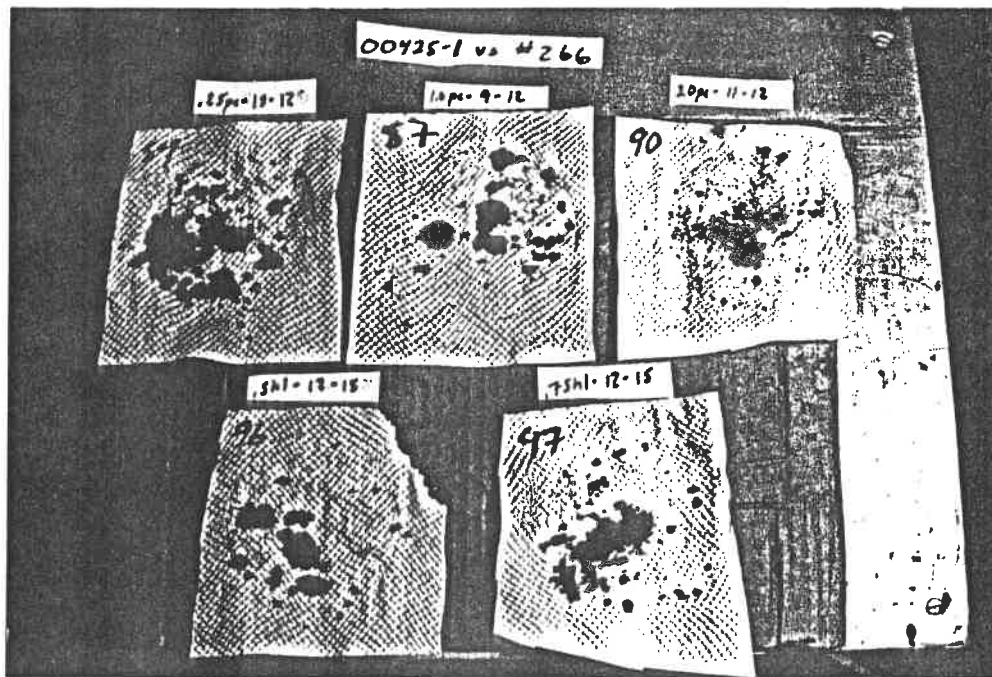
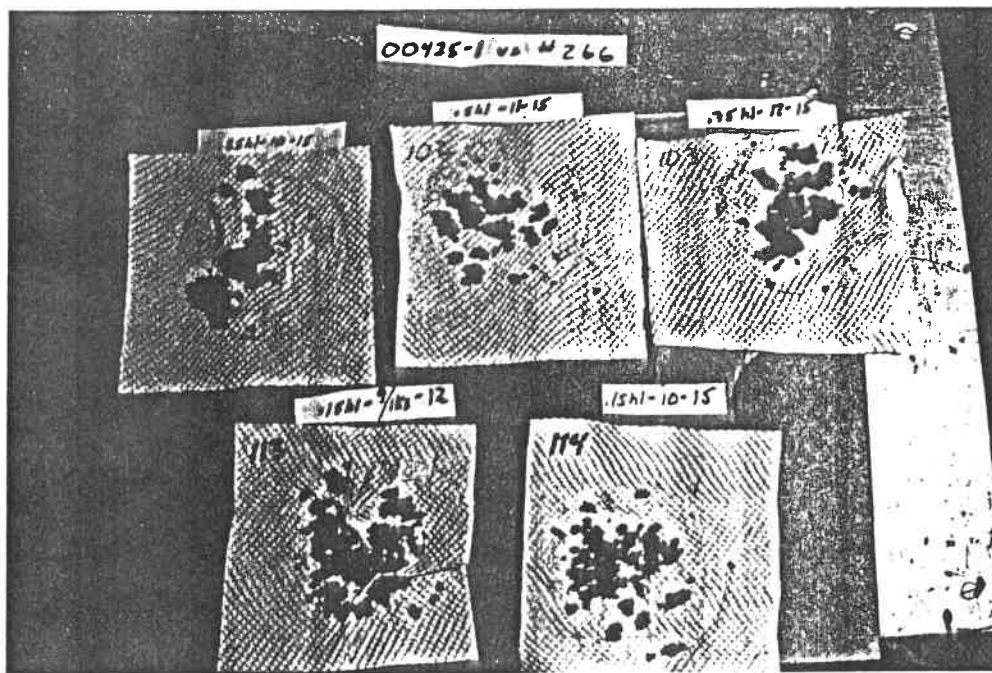
5/11/90-CRB



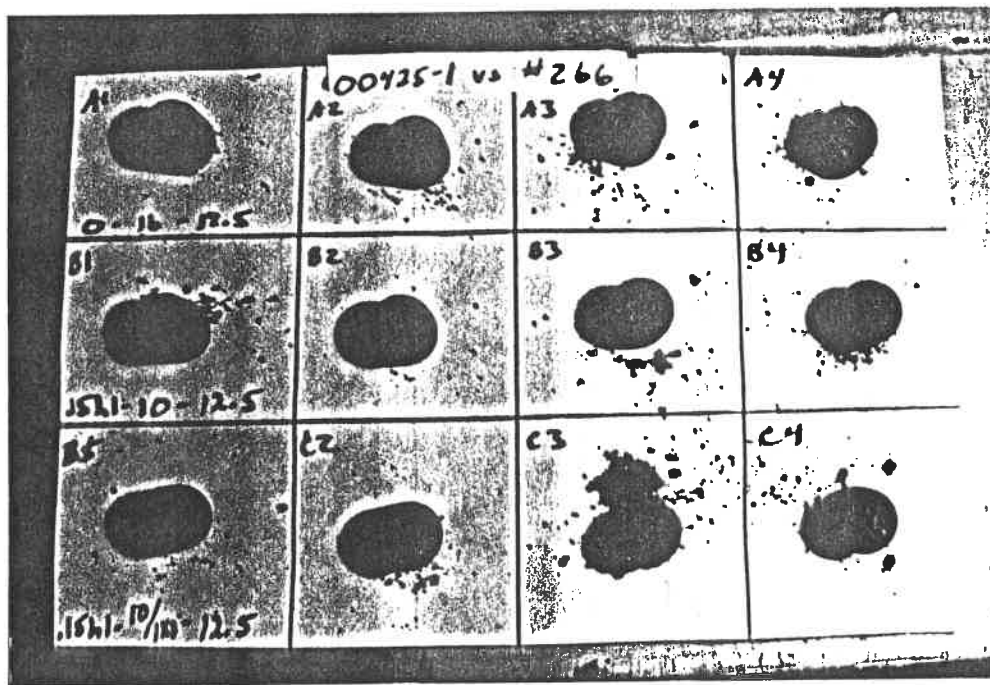
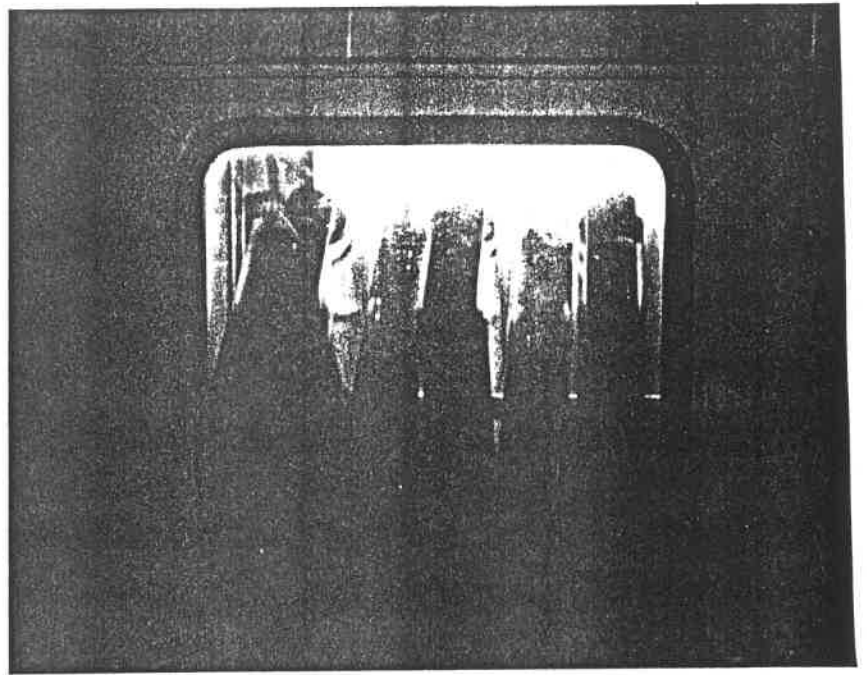
WET COHESION TEST DETERMINES THE RATE OF SET AND EARLY ROLLING TRAFFIC TIME. A POWER STEERING SIMULATOR, THE kg-cm TORQUE AT 200 kpa PRESSURE AT PEAK COHESION VALVES MAY INDICATE OPTIMUM CHEMICAL FILLER AND ADDITIVE CONTENTS.



60C (140F) CURED COHESION MEASURES HIGH TEMPERATURE COHESION AND CONFIRMS THE WET COHESION ADDITIVE CONCENTRATIONS AS WELL AS CLASSIFYING THE QUALITY OF THE SYSTEM COMPONENTS: BITUMEN, AGGREGATE, EMULSIFIER.

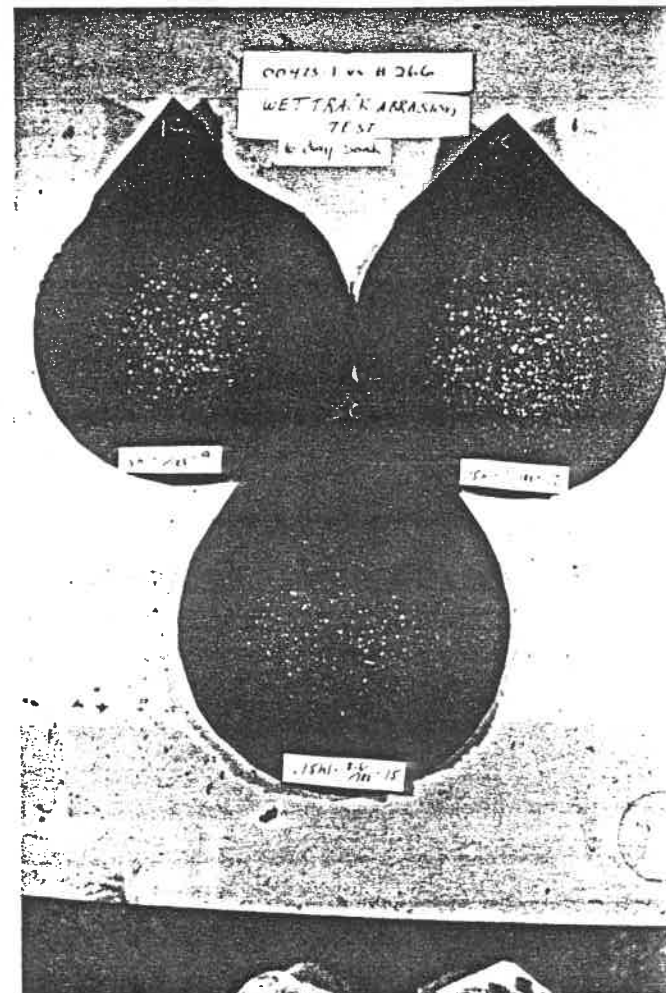
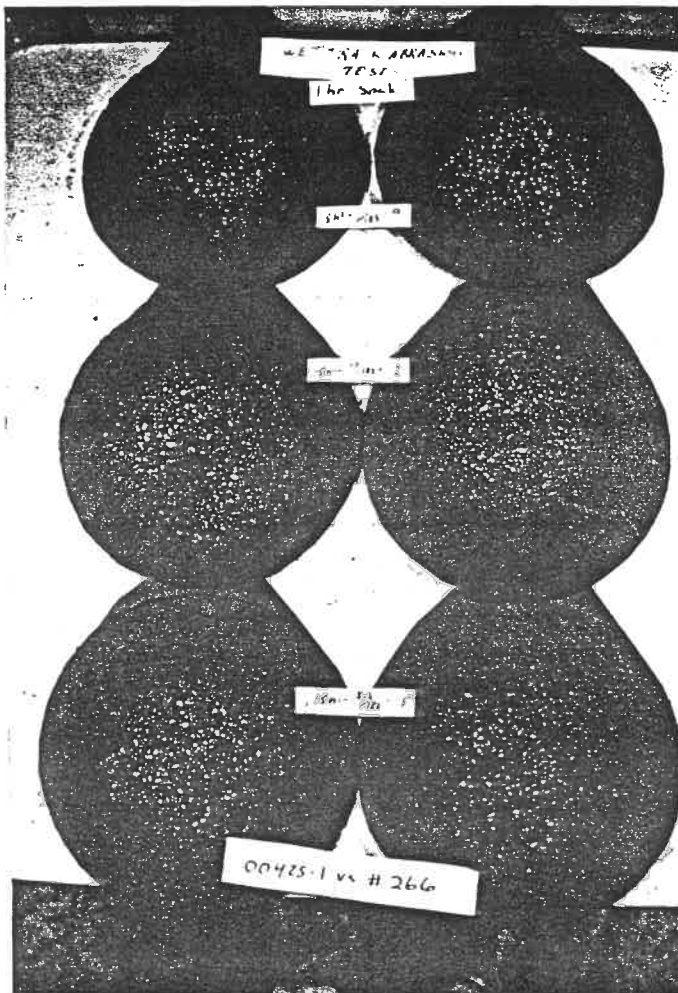


THREE MINUTE BOILING WATER ADHESION TEST.
 ABOUT 10 GRAMS OF 48-HOUR AIR CURED WET COHESION SPECIMENS
 ARE IMMERSIED IN BOILING WATER, DECANTED AND DRIED ON
 ABSORBITIVE PAPER. AN ESTIMATE OF % COATING IS MADE. HERE,
 ALL SAMPLES ARE EXCELLENT AT 98%+.

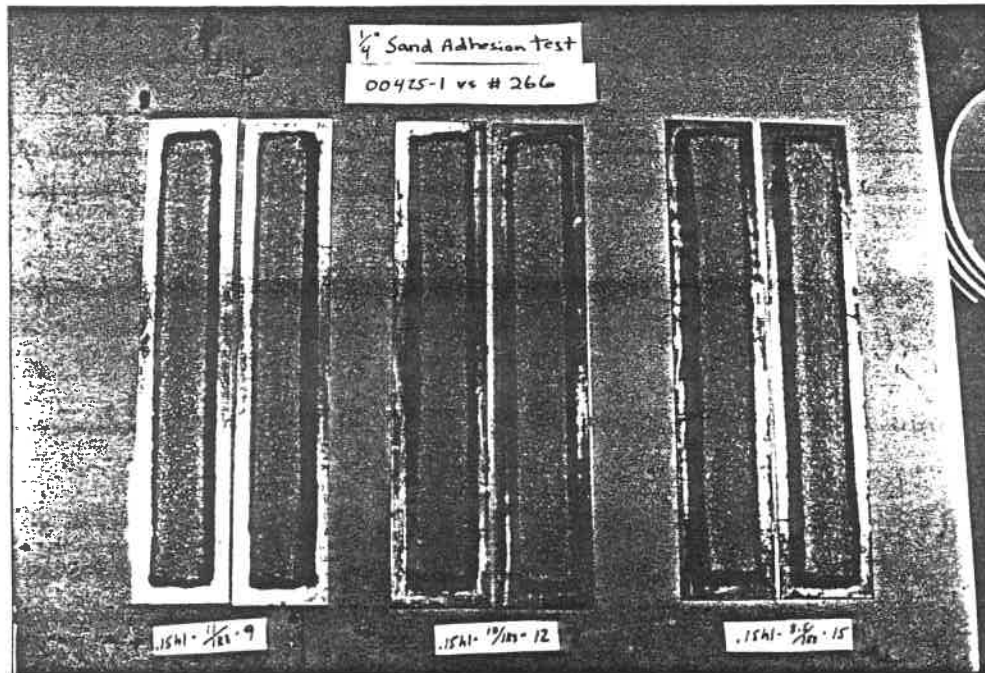
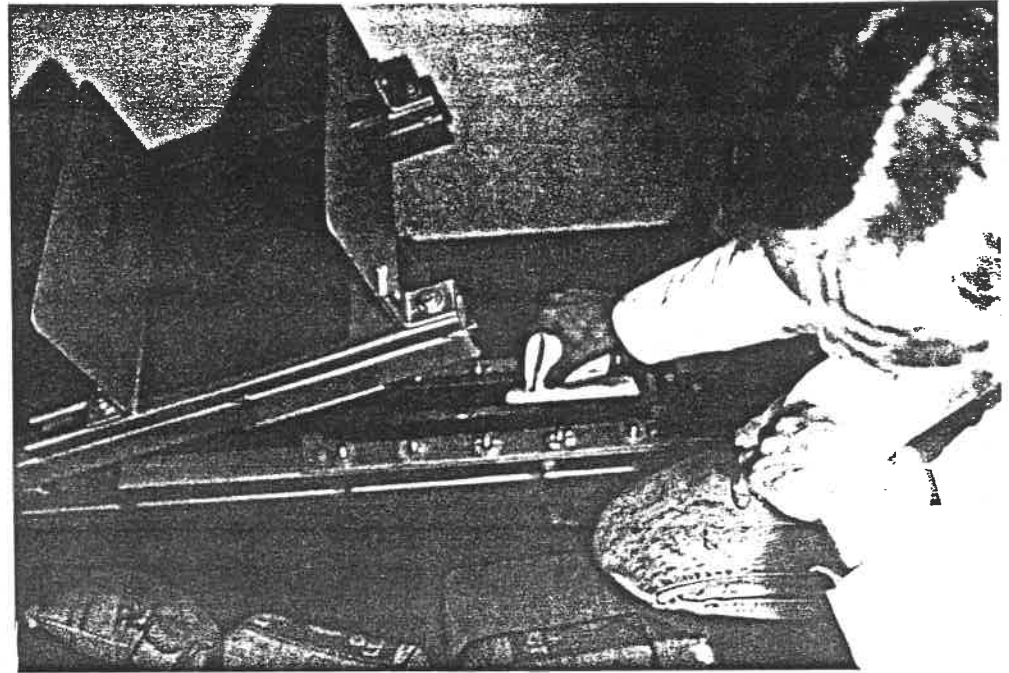
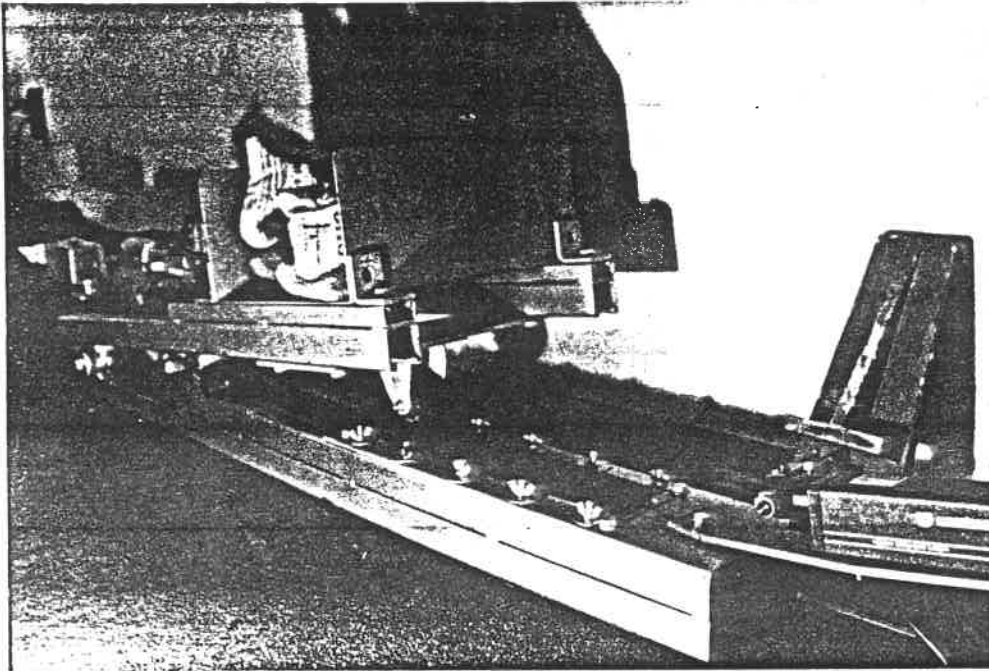


SCHULZE-BREUER-RUCK COMPATIBILITY CLASSIFICATION.

0/#10 STANDARD MIXES ARE CURED AND PRESSED IN TO QUADRUPLICATE 40-GRAM PILLS AT 140F AND 1 TON FORCE FOR 1 MINUTE. THE SPECIMENS ARE SOAKED FOR 6 DAYS, WET TUMBLED IN SHUTTLE CYLINDERS FOR 3600 CYCLES, THEN IMMERSSED IN BOILING WATER FOR 30 MINUTES. DENSITY, ABSORPTION, ABRASION LOSS, ADHESION COATING AND PERCENT REMAINING (INTEGRITY) ARE DETERMINED. COMPARATIVE GRADE POINTS ARE GIVEN. HERE, ALL SAMPLES ARE EXCELLENT AND COMPLETELY COMPATIBLE AT 12-AAA RATINGS.

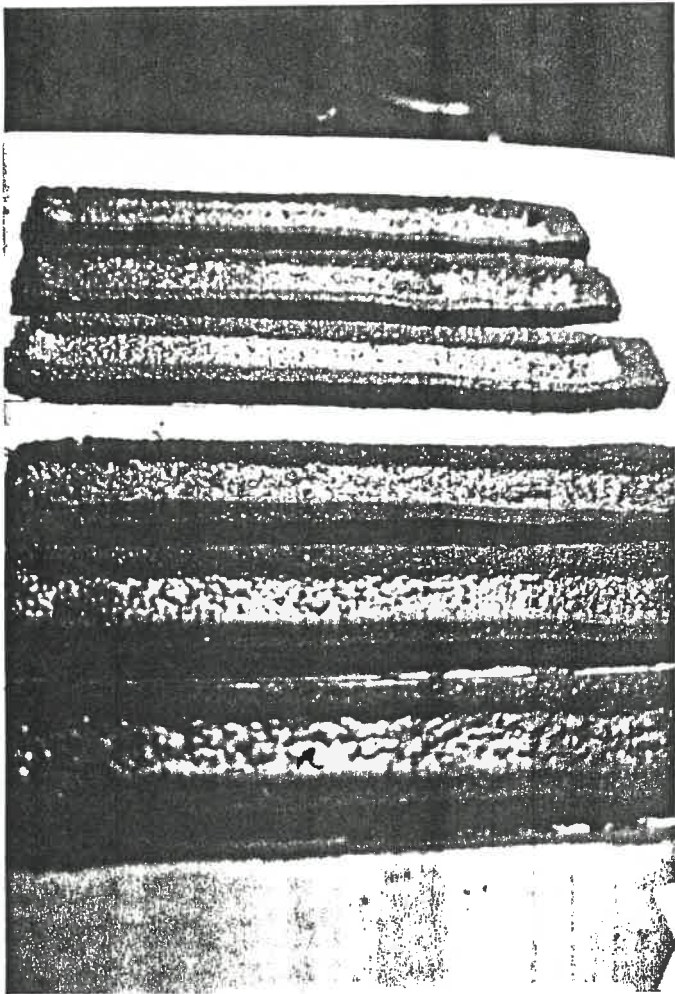
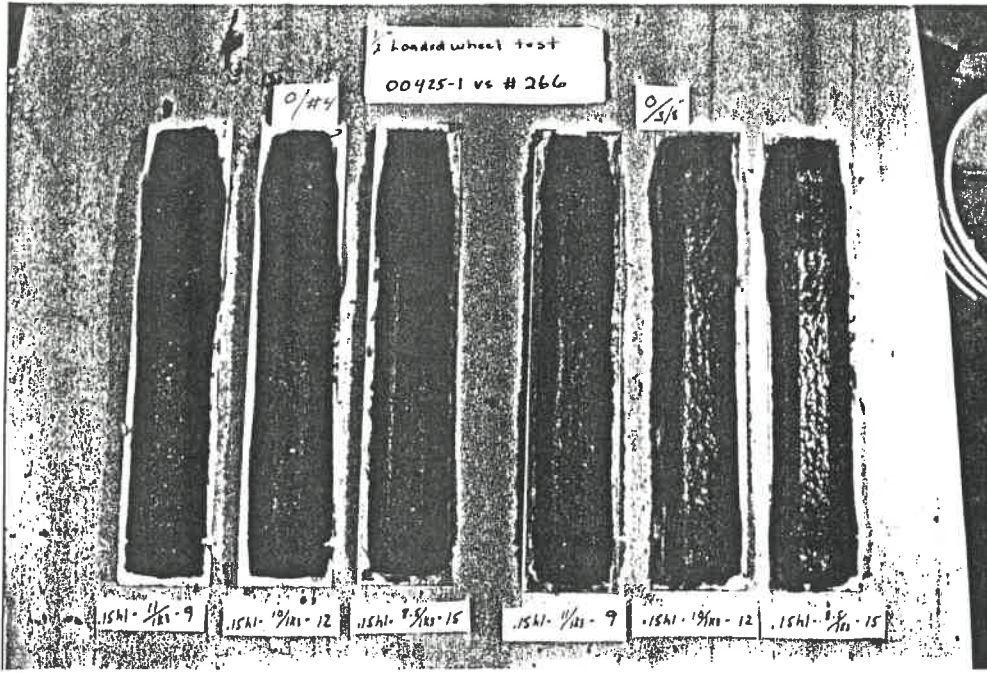


WET TRACK ABRASION TEST DETERMINES THE MINIMUM EMULSION CONTENT ASTM D 3910 PROCEDURE USES ONLY 0/#4 GRADATION AND ONE HOUR SOAK WHILE ISSA TECH BULLETIN #100 MAY ALSO USE LARGER GRADATIONS AS WELL AS A SIX-DAY SOAK. A 5-lb. WEIGHTED RUBBER HOSE SCRUBS THE IMMERSUED SPECIMEN SURFACE FOR 5-MINUTES AND THE LOSS REPORTED AS GRAMS OR GRAMS PER UNIT AREA (SQUARE FOOT OR METRES). A MAXIMUM LOSS OF 75 GRAMS PER SQ. FT., DETERMINES THE MINIMUM BITUMEN CONTENT. THE EMULSION REQUIRED TO REACH A 75 GRAM/SF & LOSS WITH A 6-DAY SOAK COMPARATIVELY CLASSIFIES THE SYSTEM; IN THIS CASE, EXCELLENT AT LESS THAN A PROJECTED 8% EMULSION!



MONOLAYER LOADED WHEEL FINE SAND
ADHESION TEST MEASURES THE RELATIVE
BITUMEN FILM THICKNESS ON THE SUR-
FACE OF A FULLY COMPACTED 0/#4 SPECIMEN.

THE MAXIMUM BITUMEN CONTENT FOR HEAVY
TRAFFIC IS DETERMINED AT 50 GRAMS PER
SQUARE FOOT ADHERED SAND.



**MULTILAYER UNCONFINED LOADED WHEEL
DISPLACEMENT TEST @ AMBIENT.**

BOTH 0/#4 AND 0-3/8" GRADATIONS,
EACH AT 3 LEVELS OF EMULSION, ARE
SUBJECTED TO 1000, 125-lb LWT CYCLES.
VERTICAL AND LATERAL DISPLACEMENTS
AND COMPACTED DENSITIES ARE DETER-
MINED. NOTE THE EXCESSIVE RICHNESS
ON THE 0-3/8" 15% AE SPECIMEN.
HERE, THE SAND ADHESION TESTS INDICATE
THE MAXIMUM PERMISSABLE AE CONTENT
AT 13%.